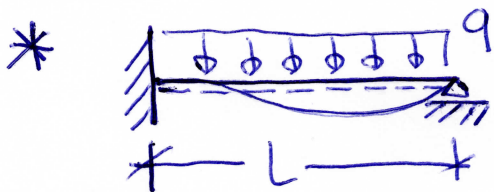


METODA SIŁ

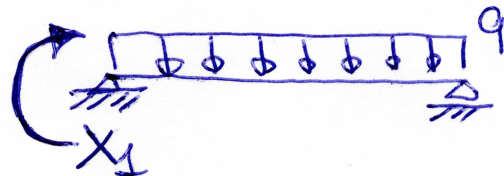
$EI = \text{const}$

$n_s = 1$

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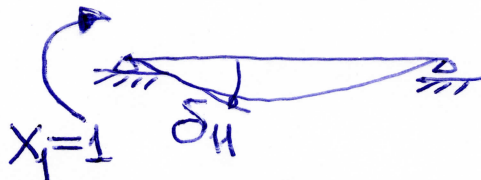
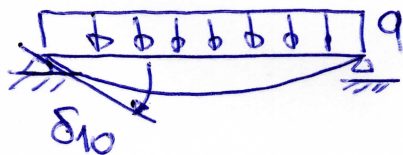


UKŁAD PODSTAWOWY METODY SIŁ

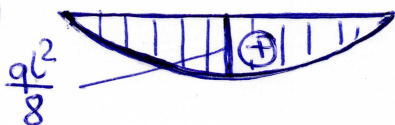


obciążenie zadane "0"

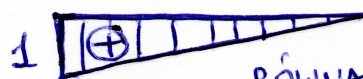
stan $X_1 = 1$



M_0



M_1



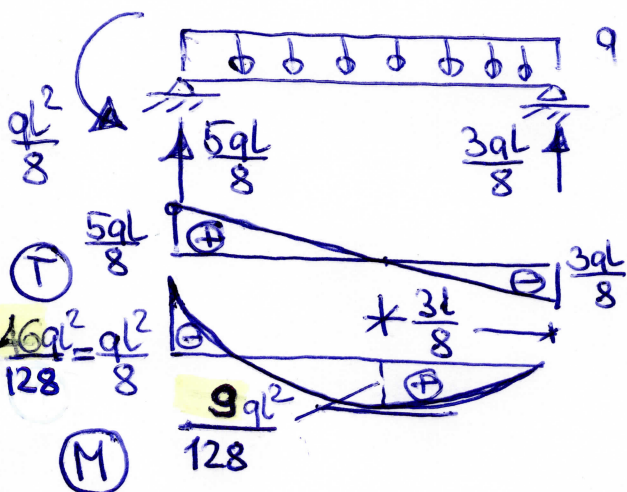
RÓWNIANIE ZGODNOŚCI PRZEMIESZCZEŃ

$$\delta_{10} = \int \frac{M_0 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{2}{3} \cdot L \cdot \frac{qL^2}{8} \cdot \frac{1}{2} \cdot 1 = \frac{qL^3}{24EI}$$

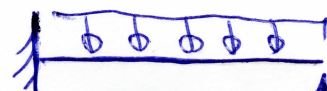
$$\delta_{11} = \int \frac{M_1 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{1}{2} \cdot L \cdot 1 \cdot \frac{2}{3} \cdot 1 = \frac{L}{3EI}$$

$$\delta_{10} + \delta_{11} X_1 = 0$$

$$X_1 = -\frac{qL^2}{8}$$

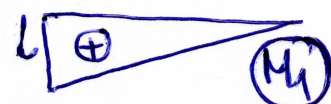
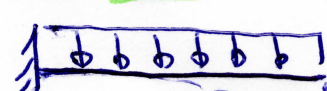


INNE PRZYJĘCIE UKŁADU PODSTAWOWEGO METODY SIŁ:



stan "0"

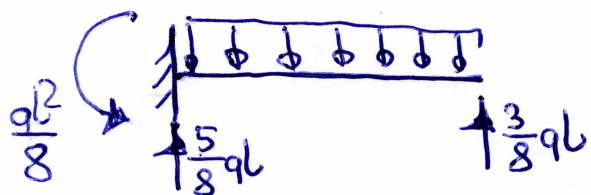
$X_1 = 1$



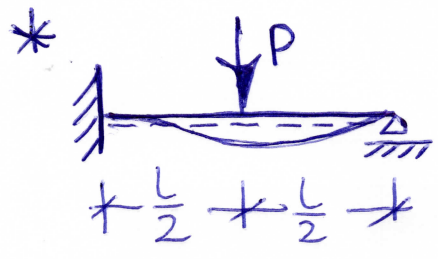
$$\delta_{10} = \int \frac{M_0 M_1}{EI} ds = \frac{-1}{EI} \cdot \frac{1}{3} \cdot L \cdot \frac{qL^2}{2} \cdot \frac{3}{4} \cdot L = -\frac{qL^4}{8EI}$$

$$\delta_{11} = \int \frac{M_1 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{1}{2} \cdot L \cdot 1 \cdot \frac{2}{3} \cdot L = \frac{L^3}{3EI}$$

$$\Rightarrow X_1 = \frac{3}{8} qL$$



spostrzeżenie: TEN SAM UKŁAD SIŁ (traktowanych tzn. obciążen zewnętrznych i reakcji podporowych), są to zatem schematy statyczne równoważne.



$EI = \text{const}$

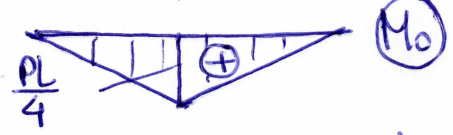
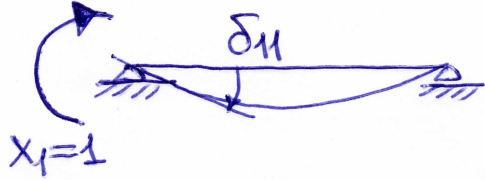
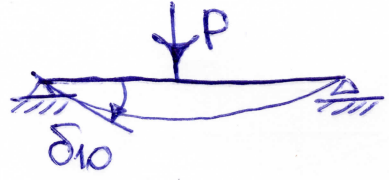
$m_s = 1$

UKŁAD PODSTAWOWY METODY SIŁ



obciążenie zadane - "0"

stan $X_1 = 1$



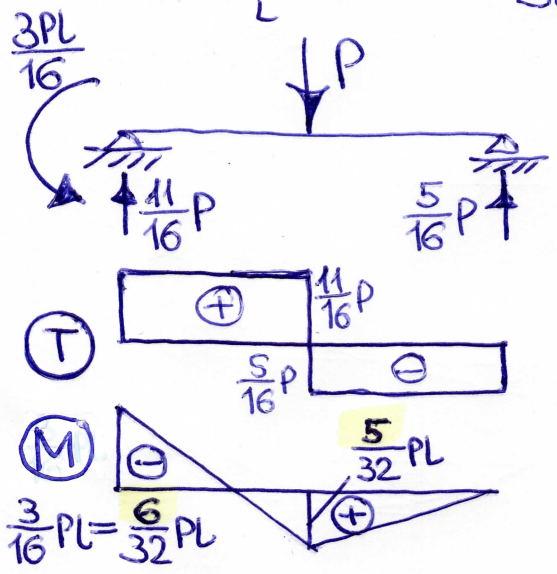
$$\delta_{10} = \int_L \frac{M_0 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{1}{2} \cdot L \cdot \frac{PL}{4} \cdot \frac{1}{2} \cdot 1 = \frac{PL^2}{16EI}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = \frac{L}{3EI}$$

RÓWNIANIE ZGODNOŚCI PRZENIESZCZEŃ

$\delta_{10} + \delta_{11} X_1 = 0$

$X_1 = -\frac{3}{16} PL$



INNY UKŁAD PODSTAWOWY METODY SIŁ:

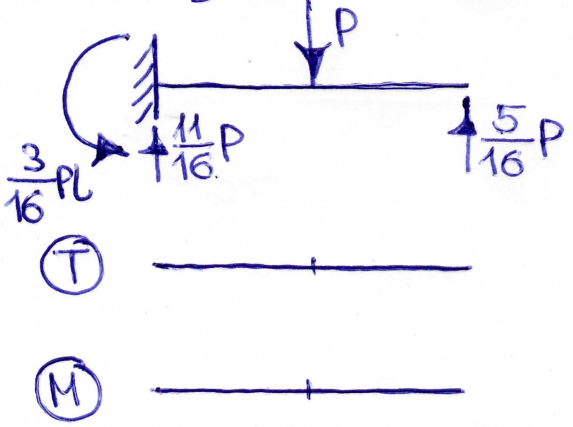
"0"

$X_1 = 1$

$$\delta_{10} = \int_L \frac{M_0 M_1}{EI} ds = -\frac{1}{EI} \cdot \frac{1}{2} \cdot \frac{L}{2} \cdot \frac{PL}{2} \cdot \frac{5}{6} L = -\frac{5}{48} \frac{PL^3}{EI}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = \frac{L^3}{3EI}$$

$X_1 = \frac{5}{16} P$

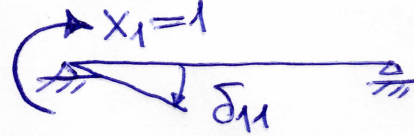
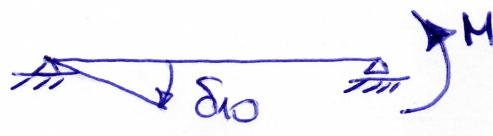


spostrzeżenie: TEN SAM UKŁAD SIŁ (traktowanych łącznie: obciążen zewnętrznych i reakcji podporowych) są to zatem schematy statycznie równoważne

UKŁAD PODSTAWOWY METODY SIŁ:

obciążenie zadane "0"

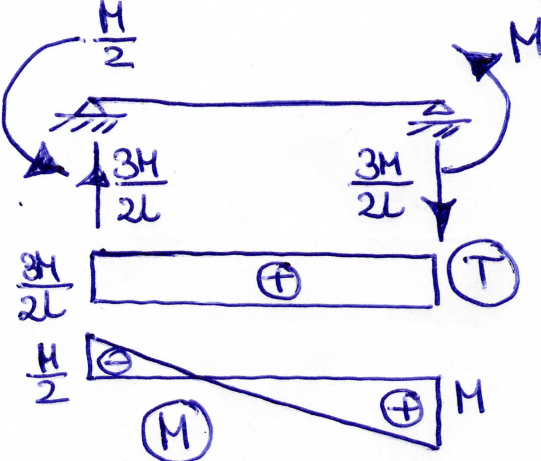
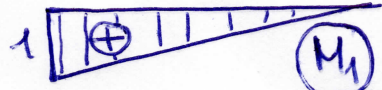
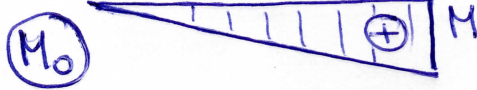
$X_1 = 1$



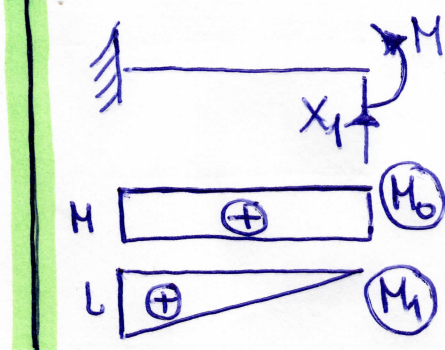
$$\delta_{10} = \int_L \frac{M_0 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{1}{2} \cdot L \cdot M \cdot \frac{1}{3} \cdot L = \frac{ML}{6EI}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = \frac{L}{3EI} \cdot M = \frac{ML}{3EI}$$

$$\delta_{10} + \delta_{11} X_1 = 0 \Rightarrow X_1 = -\frac{M}{2}$$



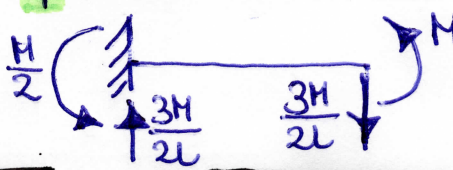
INNY UKŁAD PODSTAWOWY



$$\delta_{10} = \int_L \frac{M_0 M_1}{EI} ds = \frac{1}{EI} \cdot M \cdot L \cdot \frac{L}{2} = \frac{ML^2}{2EI}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = \frac{L^3}{3EI}$$

$$\delta_{10} + \delta_{11} X_1 = 0 \Rightarrow X_1 = -\frac{3M}{2L}$$

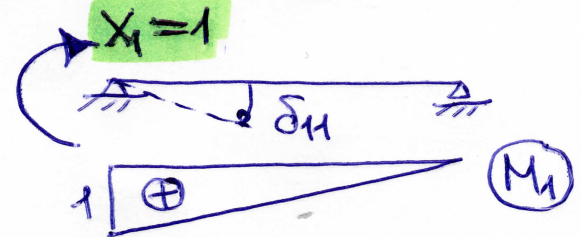


schemat statycznie równowżny

* $\Delta t = t_d - t_g = \text{const}$

DANE: $\alpha_t, \Delta t, h, EI = \text{const}$

UKŁAD PODSTAWOWY METODY SIŁ, STAN $X_1 = 1$

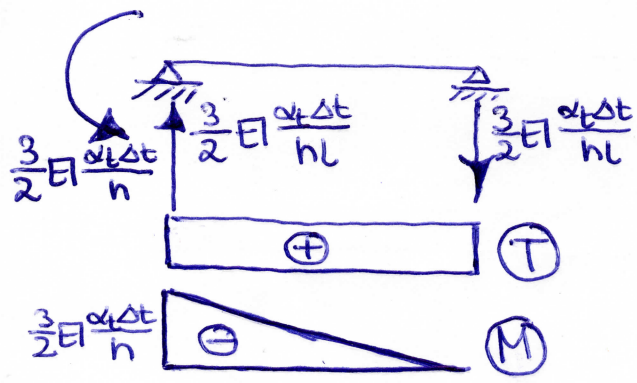


$$\delta_{10} = \frac{\alpha_t \Delta t}{h} \int_L M_1 ds = \frac{\alpha_t \Delta t}{h} \cdot \frac{L}{2}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = \frac{L}{3EI}$$

$$\delta_{10} + \delta_{11} X_1 = 0$$

$$X_1 = -\frac{3}{2} EI \frac{\alpha_t \Delta t}{h}$$

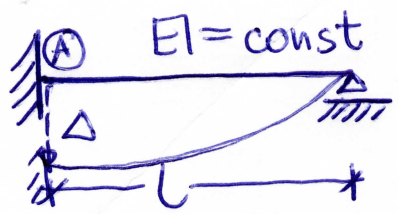


Reakcje i siły wewnętrzne pod działaniem pozostają zwrócić uwagę, gdyż UKŁAD STATYCZNIE NIETYCZNY

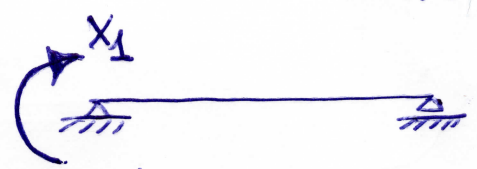
Reakcje i siły wewnętrzne są proporcjonalne do sztywności giętnej EI.

*

$n_s = 1$



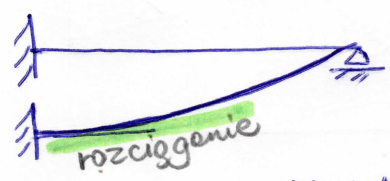
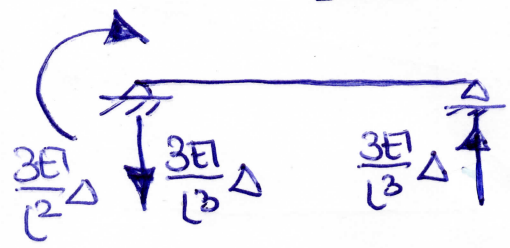
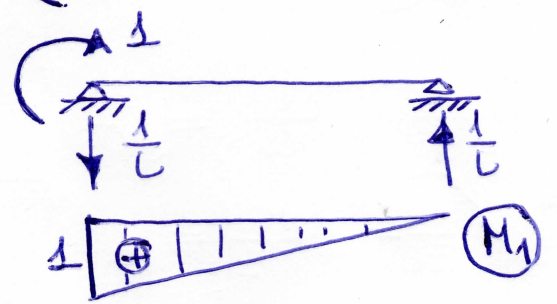
UKŁAD PODSTAWOWY METODY SIŁ



$\delta_{10} = -\Delta \cdot \frac{1}{L}$

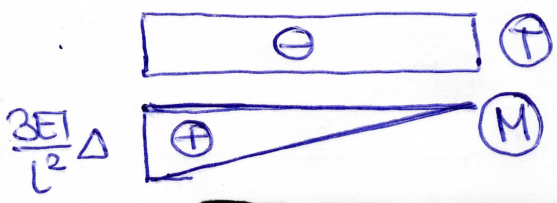
$\delta_{11} = \int \frac{M_1 M_1}{EI} ds = \frac{L}{3EI} \Rightarrow X_1 = \frac{3EI}{L^2} \Delta$

stan $X_1 = 1$



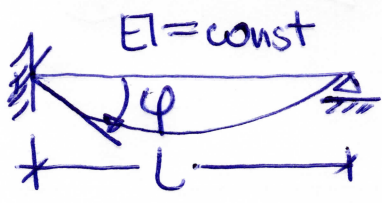
UKŁAD STATYCZNIE NIEMYZNACZALNY

także i w tym przypadku dodatkowe przemieszczenia (osiedlenie podpory) wywołuje reakcje i siły wewnętrzne, proporcjonalne do EI



*

$n_s = 1$

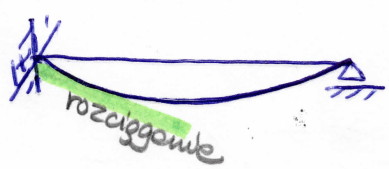
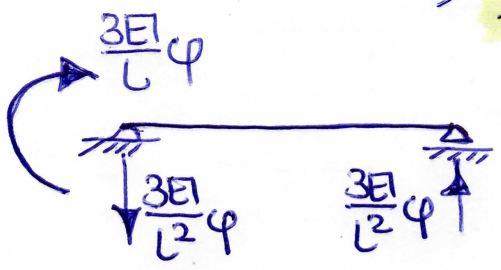
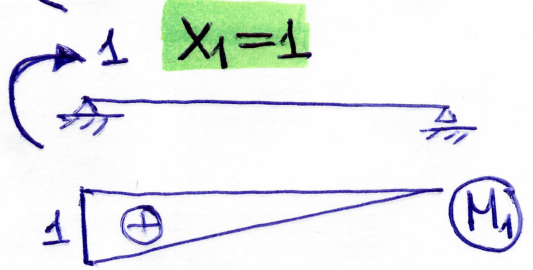


UKŁAD PODSTAWOWY METODY SIŁ



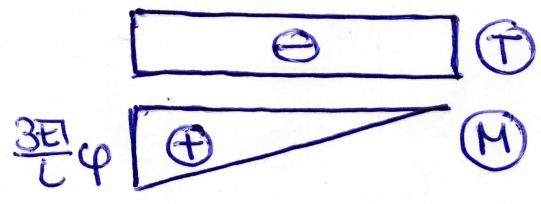
$\delta_{10} = -\varphi \cdot 1$ (ODPOWIEDNIKAMI MOMENTU UTWARDZEMA JEST $X_1 = 1$)

$\delta_{11} = \int \frac{M_1 M_1}{EI} ds = \frac{L}{3EI} \Rightarrow X_1 = \frac{3EI}{L} \varphi$

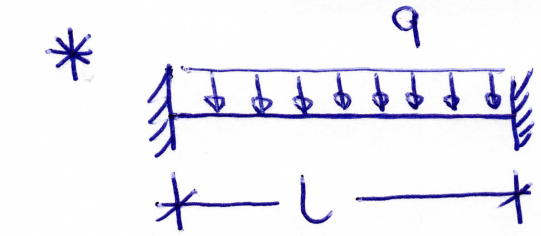


UKŁAD STATYCZNIE NIEMYZNACZALNY, dodatkowe przemieszczenia (wychylenie łętowe podpory) -

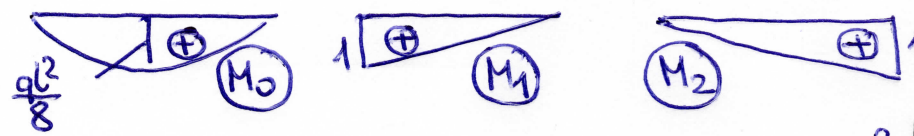
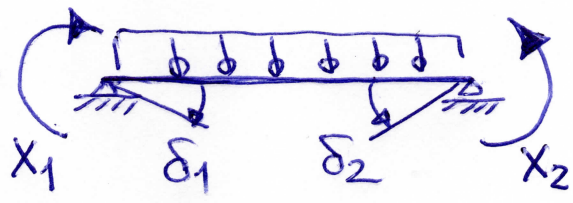
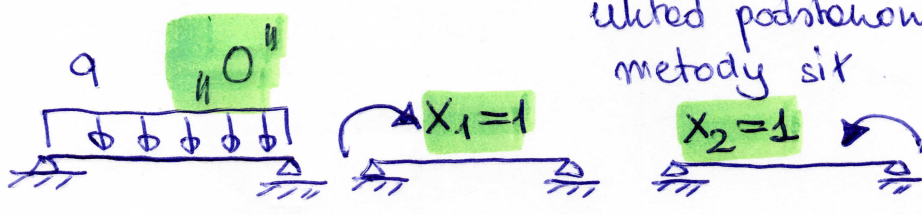
powstają reakcje i siły wewnętrzne proporcjonalne do sztywności giętnej EI



$n_s = 3$, jednak ze względu na brak sił osiowych zostają one pominięte - efektywnie $n_s = 2$



układ podstawowy metody sił



$$\delta_{10} = \delta_{20} = \frac{1}{EI} \cdot \frac{2}{3} l \cdot \frac{ql^2}{8} \cdot \frac{1}{2} = \frac{ql^3}{24EI}$$

$$\delta_{11} = \delta_{22} = \frac{l}{3EI}$$

$$\delta_{12} = \delta_{21} = \frac{l}{6EI}$$

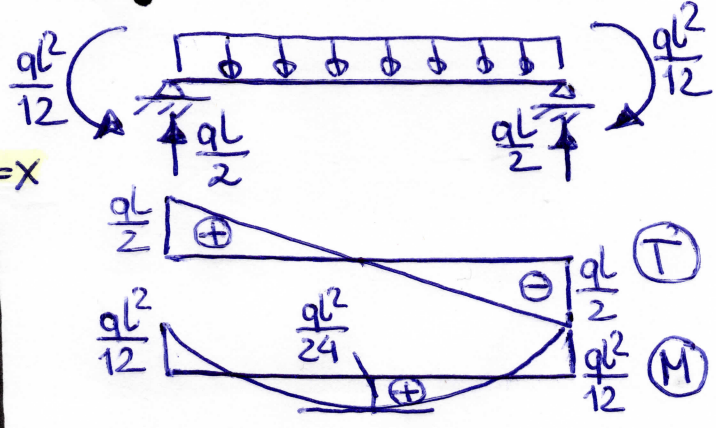
$$\delta_1 = \delta_{10} + \delta_{11}X_1 + \delta_{12}X_2 = 0 \Rightarrow 2X_1 + X_2 = -\frac{ql^2}{4}$$

$$\delta_2 = \delta_{20} + \delta_{21}X_1 + \delta_{22}X_2 = 0 \Rightarrow X_1 + 2X_2 = -\frac{ql^2}{4}$$

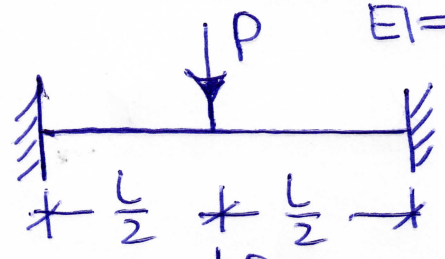
ROZWIĄZANIE: $X_1 = X_2 = -\frac{ql^2}{12}$

Symetria $\rightarrow X_1 = X_2$

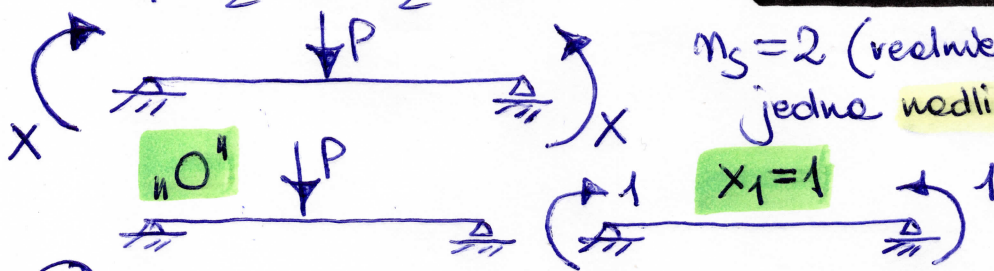
można przyjąć tzw. grupę nadliczbową $X_1 = X_2 = X$



* $EI = \text{const}$



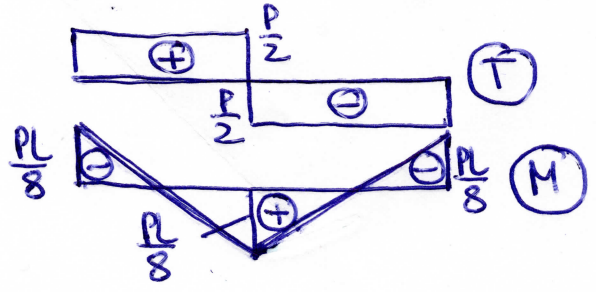
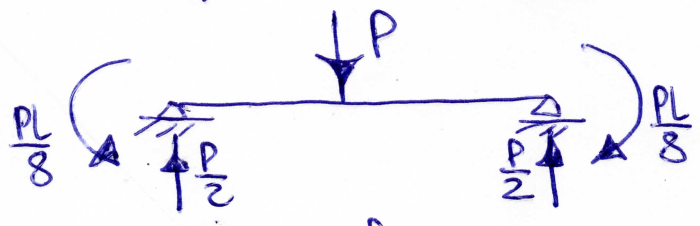
$n_s = 2$ (realnie), symetria \rightarrow jedna nadliczbowe grupowe $X_1 = X_2 = X$



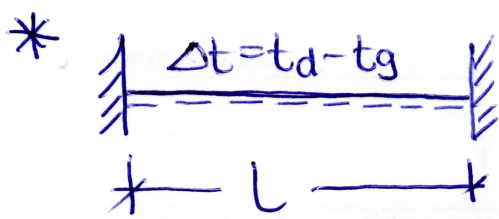
$$\delta_{10} = \frac{1}{EI} \cdot \frac{1}{2} \cdot l \cdot \frac{Pl}{4} \cdot 1 = \frac{Pl^2}{8EI}$$

$$\delta_{11} = \frac{1}{EI} \cdot 1 \cdot l \cdot 1 = \frac{l}{EI}$$

$$X_1 = -\frac{Pl}{8}$$



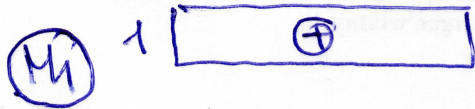
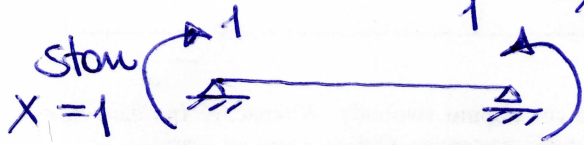
W obu powyższych przykładach symetria układu i obciążenie:
 - symetryczne układy reakcji i wykresy (M)
 - antysymetryczne wykresy (T)



dane: $EI, \alpha_t, \Delta t, h, l$

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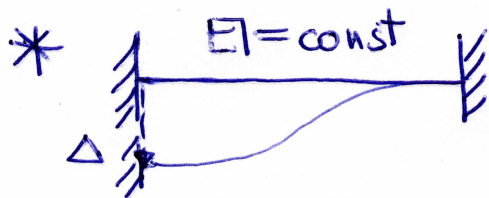
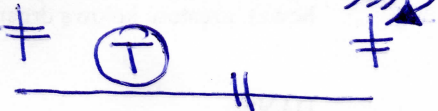
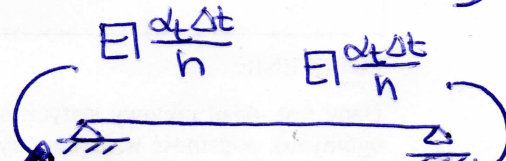
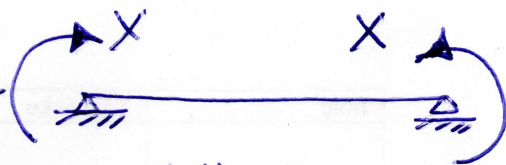
układ podstawowy,
grupowe niedliniowe
 $X_1 = X_2 = X$



$$\delta_{10} = \frac{\alpha_t \Delta t}{h} \cdot 1 \cdot l$$

$$\delta_{11} = \frac{l}{EI}$$

stąd $X_1 = -EI \frac{\alpha_t \Delta t}{h}$



$n_s = 2$ (realnie)
układ podstawowy
metody sit

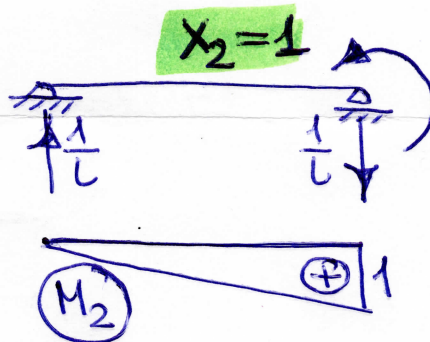
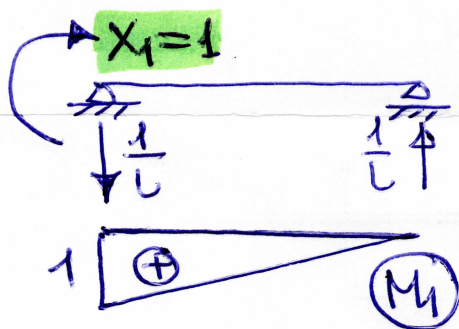


blok symetrii

$$\delta_{10} = -\frac{\Delta}{l}, \delta_{20} = \frac{\Delta}{l}$$

$$\delta_{11} = \delta_{22} = \frac{l}{3EI}$$

$$\delta_{12} = \delta_{21} = \frac{l}{6EI}$$

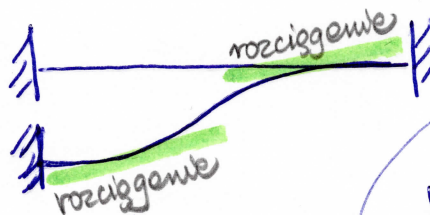
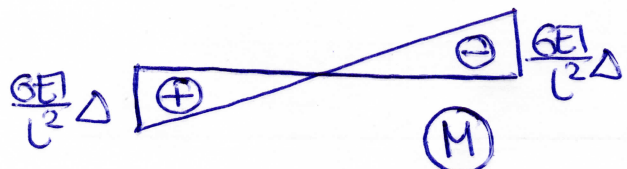
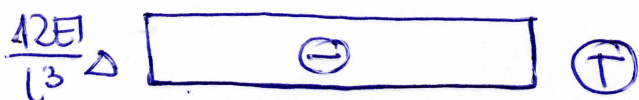
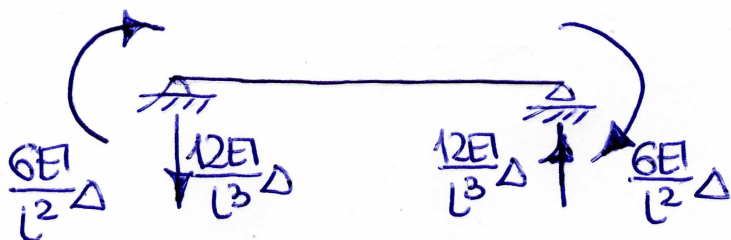


$$\delta_1 = \delta_{10} + \delta_{11} X_1 + \delta_{12} X_2 = 0 \Rightarrow 2X_1 + X_2 = \frac{6EI}{l^2} \Delta$$

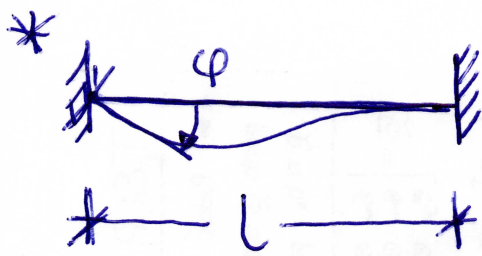
$$\delta_2 = \delta_{20} + \delta_{21} X_1 + \delta_{22} X_2 = 0 \Rightarrow X_1 + 2X_2 = -\frac{6EI}{l^2} \Delta$$

ROZWIĄZANIE: $X_1 = \frac{6EI}{l^2} \Delta$

$X_2 = -\frac{6EI}{l^2} \Delta$

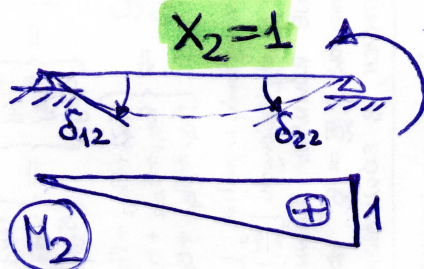
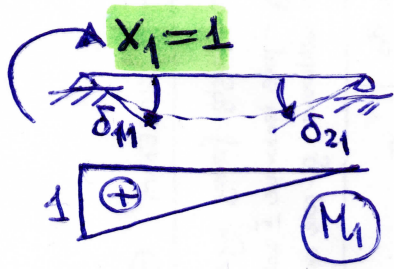


$W = 4 - 1 = 3$
 $W_1 = \frac{6EI}{l^2} \begin{vmatrix} 1 & 1 \\ 1 & 2 \end{vmatrix} = \frac{18EI}{l^2} \Delta$
 $W_2 = \frac{6EI}{l^2} \begin{vmatrix} 2 & 1 \\ 1 & -1 \end{vmatrix} = -\frac{18EI}{l^2} \Delta$
 $X_1 = \frac{6EI}{l^2} \Delta \quad X_2 = -\frac{6EI}{l^2} \Delta$



$EI = \text{const}$ $n_s = 2$ (okryty) $\bar{C}W_1 6/7$

uśredniony podstawowy metody sit



$\delta_{10} = -\varphi \cdot 1$ (nadmierzona $x_1=1$)

$\delta_{20} = 0$

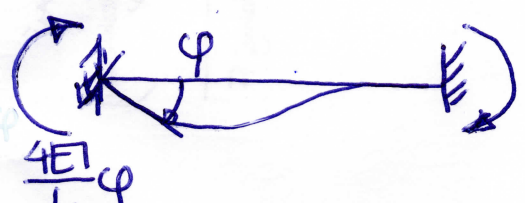
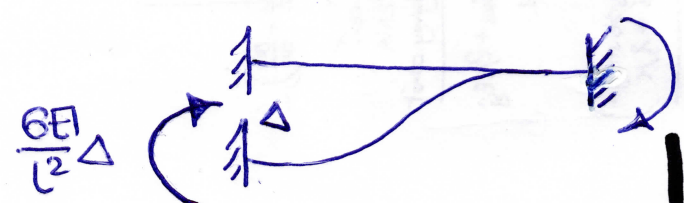
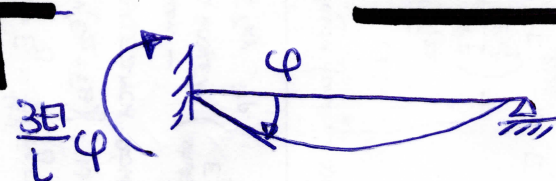
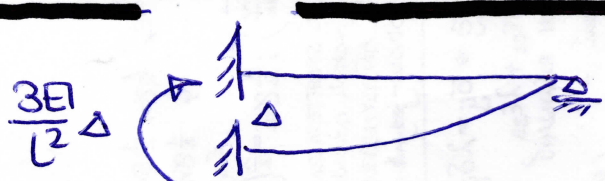
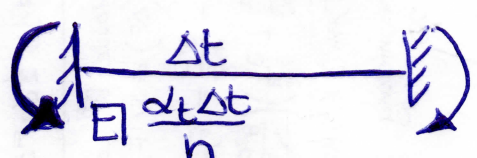
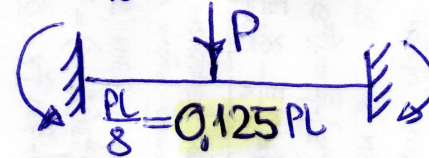
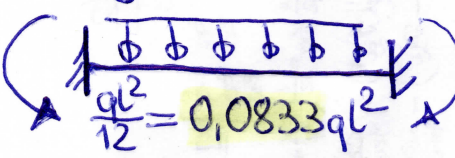
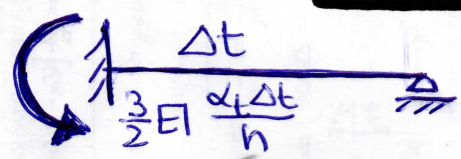
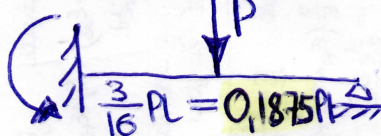
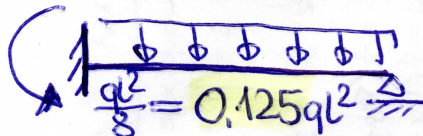
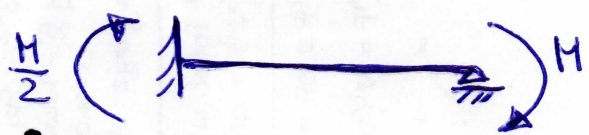
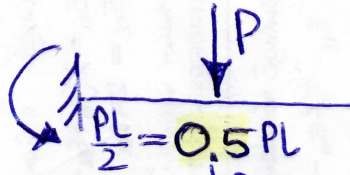
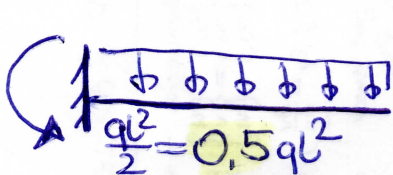
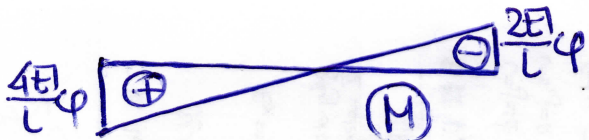
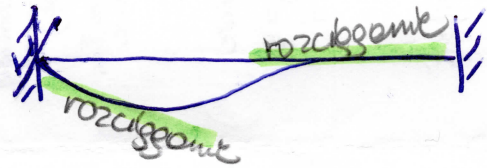
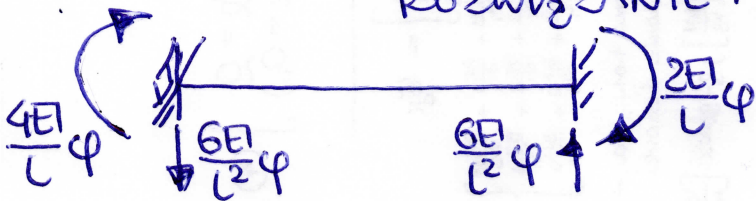
$\delta_{11} = \delta_{22} = \frac{L}{3EI}$

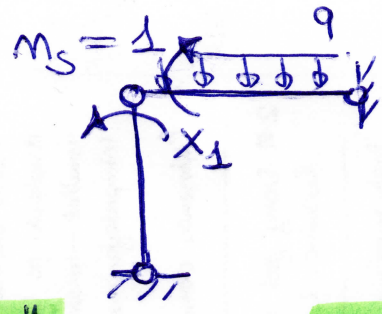
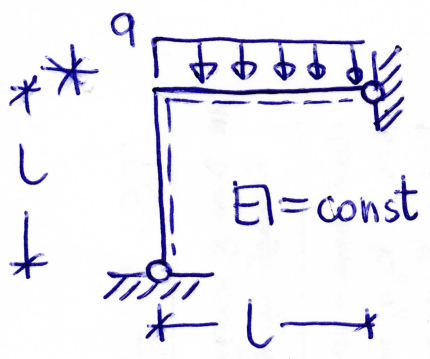
$\delta_{12} = \delta_{21} = \frac{L}{6EI}$

$\delta_{10} + \delta_{11}X_1 + \delta_{12}X_2 = 0 \Rightarrow 2X_1 + X_2 = \frac{6EI}{L}\varphi$

$\delta_{20} + \delta_{21}X_1 + \delta_{22}X_2 = 0 \Rightarrow X_1 + 2X_2 = 0$

ROZWIĄZANIE: $X_1 = \frac{4EI}{L}\varphi$, $X_2 = -\frac{2EI}{L}\varphi$

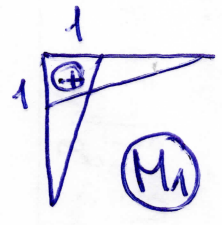
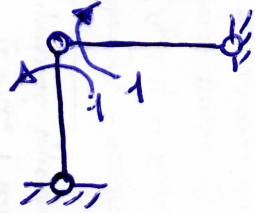
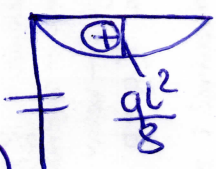
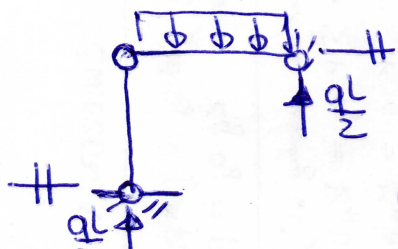




użył podsternomy metody sit

40^4

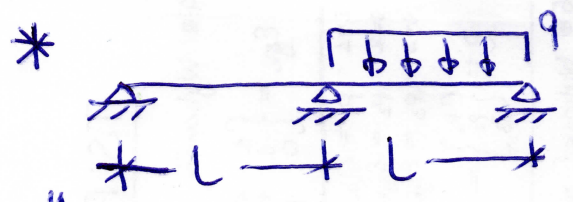
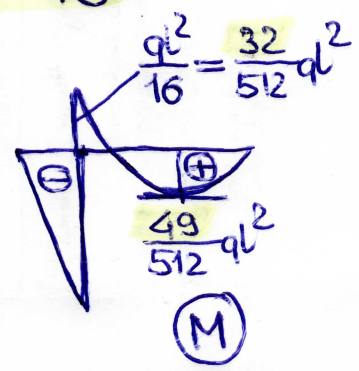
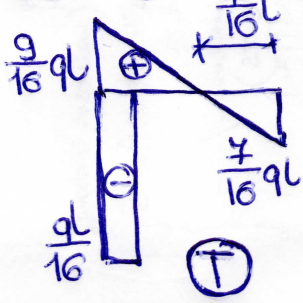
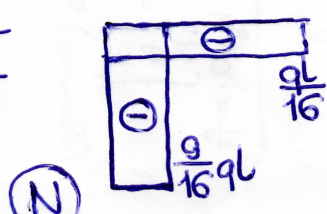
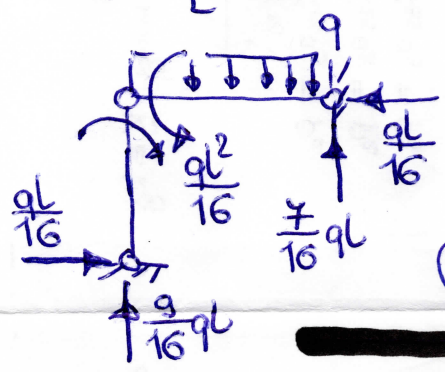
$X_1 = 1$



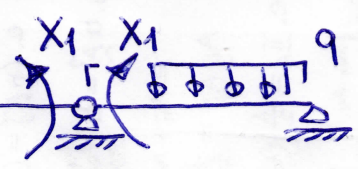
$$\delta_{10} = \int_L \frac{M_0 M_1}{EI} ds = \frac{1}{EI} \cdot \frac{2}{3} \cdot L \cdot \frac{ql^2}{8} \cdot \frac{1}{2} \cdot 1 = \frac{ql^3}{24EI}$$

$$\delta_{11} = \int_L \frac{M_1 M_1}{EI} ds = 2 \cdot \frac{1}{EI} \cdot \frac{1}{2} \cdot L \cdot 1 \cdot \frac{2}{3} \cdot 1 = \frac{2L}{3EI}$$

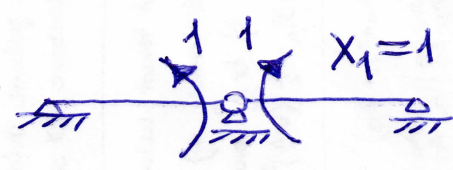
$X_1 = -\frac{ql^2}{16}$



EI = const

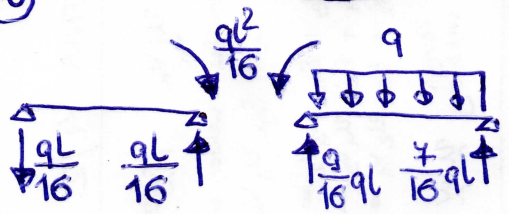
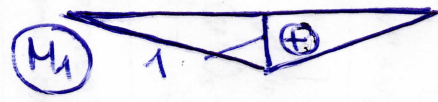
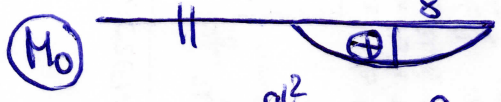


użył podsternomy metody sit

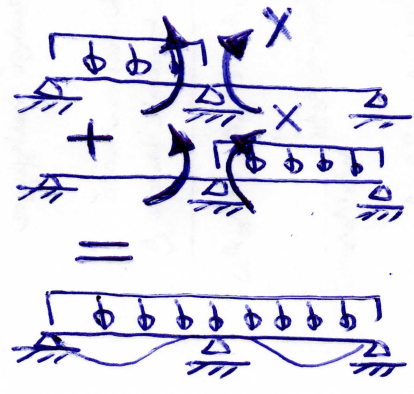
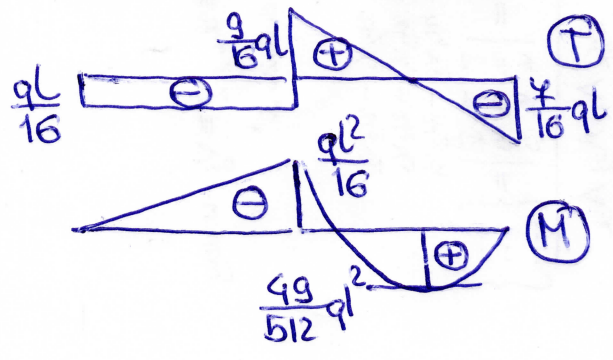


$$\delta_{10} = \frac{ql^3}{24EI} \quad \delta_{11} = \frac{2L}{3EI}$$

$X_1 = -\frac{ql^2}{16}$



SPOSTRZEŻENIE: proz wyłnesem (N) rozciąganie wamy i belli identyczne



$X = -\frac{ql^2}{16}$

